

Solutions to Chapter 3 Problems

S.3.1

Initially the stress function, ϕ , must be expressed in terms of Cartesian coordinates. Thus, from the equation of a circle of radius, a , and having the origin of its axes at its centre.

$$\phi = k(x^2 + y^2 - a^2) \quad (\text{i})$$

From Eqs (3.4) and (3.11)

$$\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} = F = -2G \frac{d\theta}{dz} \quad (\text{ii})$$

Differentiating Eq. (i) and substituting in Eq. (ii)

$$4k = -2G \frac{d\theta}{dz}$$

or

$$k = -\frac{1}{2}G \frac{d\theta}{dz} \quad (\text{iii})$$

From Eq. (3.8)

$$T = 2 \iint \phi \, dx \, dy$$

i.e.

$$T = -G \frac{d\theta}{dz} \left[\iint_A x^2 \, dx \, dy + \iint_A y^2 \, dx \, dy - a^2 \iint_A dx \, dy \right] \quad (\text{iv})$$

where $\iint_A x^2 \, dx \, dy = I_y$, the second moment of area of the cross-section about the y axis; $\iint_A y^2 \, dx \, dy = I_x$, the second moment of area of the cross-section about the x axis and $\iint_A dx \, dy = A$, the area of the cross-section. Thus, since $I_y = \pi a^4/4$, $I_x = \pi a^4/4$ and $A = \pi a^2$ Eq. (iv) becomes

$$T = G \frac{d\theta}{dz} \frac{\pi a^4}{2}$$

or

$$\frac{d\theta}{dz} = \frac{2T}{G\pi a^4} = \frac{T}{GI_p} \quad (\text{v})$$

From Eqs (3.2) and (v)

$$\tau_{zy} = -\frac{\partial \phi}{\partial x} = -2kx = G \frac{d\theta}{dz} x = \frac{Tx}{I_p} \quad (\text{vi})$$

and

$$\tau_{zx} = \frac{\partial \phi}{\partial y} = 2ky = -G \frac{d\theta}{dz} y = -\frac{T y}{I_p} \quad (\text{vii})$$

Substituting for τ_{zy} and τ_{zx} from Eqs (vi) and (vii) in the second of Eqs (3.15)

$$\tau_{zs} = \frac{T}{I_p}(xl + ym) \quad (\text{viii})$$

in which, from Eqs (3.6)

$$l = \frac{dy}{ds} \quad m = -\frac{dx}{ds}$$

Suppose that the bar of Fig. 3.2 is circular in cross-section and that the radius makes an angle α with the x axis. Then.

$$m = \sin \alpha \quad \text{and} \quad l = \cos \alpha$$

Also, at any radius, r

$$y = r \sin \alpha \quad x = r \cos \alpha$$

Substituting for x, l, y and m in Eq. (viii) gives

$$\tau_{zs} = \frac{Tr}{I_p} (= \tau)$$

Now substituting for τ_{zx}, τ_{zy} and $d\theta/dz$ from Eqs (vii), (vi) and (v) in Eqs (3.10)

$$\frac{\partial w}{\partial x} = -\frac{T y}{GI_p} + \frac{T y}{GI_p} = 0 \quad (\text{ix})$$

$$\frac{\partial w}{\partial y} = \frac{T x}{GI_p} - \frac{T x}{GI_p} = 0 \quad (\text{x})$$

The possible solutions of Eqs (ix) and (x) are $w = 0$ and $w = \text{constant}$. The latter solution implies a displacement of the whole bar along the z axis which, under the given loading, cannot occur. Therefore, the first solution applies, i.e. the warping is zero at all points in the cross-section.

The stress function, ϕ , defined in Eq. (i) is constant at any radius, r , in the cross-section of the bar so that there are no shear stresses acting across such a boundary. Thus, the material contained within this boundary could be removed without affecting the stress distribution in the outer portion. Therefore, the stress function could be used for a hollow bar of circular cross-section.

S.3.2

In S.3.1 it has been shown that the warping of the cross-section of the bar is everywhere zero. Then, from Eq. (3.17) and since $d\theta/dz \neq 0$

$$\psi(x, y) = 0 \quad (\text{i})$$

This warping function satisfies Eq. (3.20). Also Eq. (3.21) reduces to

$$xm - yl = 0 \quad (\text{ii})$$

On the boundary of the bar $x = al$, $y = am$ so that Eq. (ii), i.e. Eq. (3.21), is satisfied.

Since $\psi = 0$, Eq. (3.23) for the torsion constant reduces to

$$J = \iint_A x^2 dx dy + \iint_A y^2 dx dy = I_p$$

Therefore, from Eq. (3.12)

$$T = GI_p \frac{d\theta}{dz}$$

as in S.3.1.

From Eqs (3.19)

$$\tau_{zx} = G \frac{d\theta}{dz} (-y) = -\frac{Ty}{I_p}$$

and

$$\tau_{zy} = G \frac{d\theta}{dz} (x) = \frac{Tx}{I_p}$$

which are identical to Eqs (vii) and (vi) in S.3.1. Hence

$$\tau_{zs} = \tau = \frac{Tr}{I_p}$$

as in S.3.1.

S.3.3

Since $\psi = kxy$, Eq. (3.20) is satisfied.

Substituting for ψ in Eq. (3.21)

$$(kx + x)m + (ky - y)l = 0$$

or, from Eqs (3.6)

$$-x(k+1)\frac{dx}{ds} + y(k-1)\frac{dy}{ds} = 0$$

or

$$\frac{d}{ds} \left[-\frac{x^2}{2}(k+1) + \frac{y^2}{2}(k-1) \right] = 0$$

so that

$$-\frac{x^2}{2}(k+1) + \frac{y^2}{2}(k-1) = \text{constant on the boundary of the bar}$$

Rearranging

$$x^2 + \left(\frac{1-k}{1+k} \right) y^2 = \text{constant} \quad (\text{i})$$

Also, the equation of the elliptical boundary of the bar is

$$\frac{x^2}{a^2} + \frac{y^2}{b^2} = 1$$

or

$$x^2 + \frac{a^2}{b^2} y^2 = a^2 \quad (\text{ii})$$

Comparing Eqs (i) and (ii)

$$\frac{a^2}{b^2} = \left(\frac{1-k}{1+k} \right)$$

from which

$$k = \frac{b^2 - a^2}{a^2 + b^2} \quad (\text{iii})$$

and

$$\psi = \frac{b^2 - a^2}{a^2 + b^2} xy \quad (\text{iv})$$

Substituting for ψ in Eq. (3.23) gives the torsion constant, J , i.e.

$$J = \iint_A \left[\left(\frac{b^2 - a^2}{a^2 + b^2} + 1 \right) x^2 - \left(\frac{b^2 - a^2}{a^2 + b^2} - 1 \right) y^2 \right] dx dy \quad (\text{v})$$

Now $\iint_A x^2 dx dy = I_y = \pi a^3 b / 4$ for an elliptical cross-section. Similarly $\iint_A y^2 dx dy = I_x = \pi a b^3 / 4$. Equation (v) therefore simplifies to

$$J = \frac{\pi a^3 b^3}{a^2 + b^2} \quad (\text{vi})$$

which are identical to Eq. (v) of Example 3.1.

From Eq. (3.22) the rate of twist is

$$\frac{d\theta}{dz} = \frac{T(a^2 + b^2)}{G\pi a^3 b^3} \quad (\text{vii})$$

The shear stresses are obtained from Eqs (3.19), i.e.

$$\tau_{zx} = \frac{GT(a^2 + b^2)}{G\pi a^3 b^3} \left[\left(\frac{b^2 - a^2}{a^2 + b^2} \right) y - y \right]$$

so that

$$\tau_{zx} = -\frac{2Ty}{\pi ab^3}$$

and

$$\tau_{zy} = \frac{GT(a^2 + b^2)}{G\pi a^3 b^3} \left[\left(\frac{b^2 - a^2}{a^2 + b^2} \right) x + x \right]$$

i.e.

$$\tau_{zy} = \frac{2Tx}{\pi a^3 b}$$

which are identical to Eq. (vi) of Example 3.1.

From Eq. (3.17)

$$w = \frac{T(a^2 + b^2)}{G\pi a^3 b^3} \left(\frac{b^2 - a^2}{a^2 + b^2} \right) xy$$

i.e.

$$w = \frac{T(b^2 - a^2)}{G\pi a^3 b^3} xy \quad (\text{compare with Eq. (viii) of Example 3.1})$$

S.3.4

The stress function is

$$\phi = -G \frac{d\theta}{dz} \left[\frac{1}{2}(x^2 + y^2) - \frac{1}{2a}(x^3 - 3xy^2) - \frac{2}{27}a^2 \right] \quad (i)$$

Differentiating Eq. (i) twice with respect to x and y in turn gives

$$\frac{\partial^2 \phi}{\partial x^2} = -G \frac{d\theta}{dz} \left(1 - \frac{3x}{a} \right)$$

$$\frac{\partial^2 \phi}{\partial y^2} = -G \frac{d\theta}{dz} \left(1 + \frac{3x}{a} \right)$$

Therefore

$$\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} = -2G \frac{d\theta}{dz} = \text{constant}$$

and Eq. (3.4) is satisfied.

Further

$$\text{on AB, } x = \frac{-a}{3} \quad y = y$$

$$\text{on BC, } y = \frac{-x}{\sqrt{3}} + \frac{2a}{3\sqrt{3}}$$

$$\text{on AC, } y = \frac{x}{\sqrt{3}} - \frac{2a}{3\sqrt{3}}$$

Substituting these expressions in turn in Eq. (i) gives

$$\phi_{AB} = \phi_{BC} = \phi_{AC} = 0$$

so that Eq. (i) satisfies the condition $\phi = 0$ on the boundary of the triangle.

From Eqs (3.2) and (i)

$$\tau_{zy} = -\frac{\partial\phi}{\partial x} = G\frac{d\theta}{dz} \left(x - \frac{3x^2}{2a} + \frac{3y^2}{2a} \right) \quad (\text{ii})$$

and

$$\tau_{zx} = \frac{\partial\phi}{\partial y} = -G\frac{d\theta}{dz} \left(y + \frac{3xy}{a} \right) \quad (\text{iii})$$

At each corner of the triangular section $\tau_{zy} = \tau_{zx} = 0$. Also, from antisymmetry, the distribution of shear stress will be the same along each side. For AB, where $x = -a/3$ and $y = y$, Eqs (ii) and (iii) become

$$\tau_{zy} = G\frac{d\theta}{dz} \left(-\frac{a}{2} + \frac{3y^2}{2a} \right) \quad (\text{iv})$$

and

$$\tau_{zx} = 0 \quad (\text{v})$$

From Eq. (iv) the maximum value of τ_{zy} occurs at $y = 0$ and is

$$\tau_{zy}(\text{max}) = -\frac{Ga}{2} \frac{d\theta}{dz} \quad (\text{vi})$$

The distribution of shear stress along the x axis is obtained from Eqs (ii) and (iii) in which $x = x$, $y = 0$, i.e.

$$\tau_{zy} = G\frac{d\theta}{dz} \left(x - \frac{3x^2}{2a} \right) \quad (\text{vii})$$

$$\tau_{zx} = 0$$

From Eq. (vii) τ_{zy} has a mathematical maximum at $x = +a/3$ which gives

$$\tau_{zy} = \frac{Ga}{6} \frac{d\theta}{dz} \quad (\text{viii})$$

which is less than the value given by Eq. (vi). Thus the maximum value of shear stress in the section is $(-Ga/2)d\theta/dz$.

The rate of twist may be found by substituting for ϕ from Eq. (i) in (3.8).

Thus

$$T = -2G \frac{d\theta}{dz} \iint \left[\frac{1}{2}(x^2 + y^2) - \frac{1}{2a}(x^3 - 3xy^2) - \frac{2}{27}a^2 \right] dx dy \quad (\text{ix})$$

The equation of the side AC of the triangle is $y = (x - 2a/3)/\sqrt{3}$ and that of BC, $y = -(x - 2a/3)/\sqrt{3}$. Equation (ix) then becomes

$$T = -2G \frac{d\theta}{dz} \int_{-a/3}^{2a/3} \int_{(x-2a/3)/\sqrt{3}}^{-(x-2a/3)/\sqrt{3}} \left[\frac{1}{2}(x^2 + y^2) - \frac{1}{2a}(x^3 - 3xy^2) - \frac{2}{27}a^2 \right] dx dy$$

which gives

$$T = \frac{Ga^4}{15\sqrt{3}} \frac{d\theta}{dz}$$

so that

$$\frac{d\theta}{dz} = \frac{15\sqrt{3}T}{Ga^4} \quad (\text{x})$$

From the first of Eqs (3.10)

$$\frac{\partial w}{\partial x} = \frac{\tau_{zx}}{G} + \frac{d\theta}{dz} y$$

Substituting for τ_{zx} from Eq. (iii)

$$\frac{\partial w}{\partial x} = -\frac{d\theta}{dz} \left(y + \frac{3xy}{a} - y \right)$$

i.e.

$$\frac{\partial w}{\partial x} = -\frac{3xy}{a} \frac{d\theta}{dz}$$

whence

$$w = -\frac{3x^2y}{2a} \frac{d\theta}{dz} + f(y) \quad (\text{xi})$$

Similarly from the second of Eqs (3.10)

$$w = -\frac{3x^2y}{2a} \frac{d\theta}{dz} + \frac{y^3}{2a} \frac{d\theta}{dz} + f(x) \quad (\text{xii})$$

Comparing Eqs (xi) and (xii)

$$f(x) = 0 \quad \text{and} \quad f(y) = \frac{y^3}{2a} \frac{d\theta}{dz}$$

Hence

$$w = \frac{1}{2a} \frac{d\theta}{dz} (y^3 - 3x^2y).$$

S.3.5

The torsion constant, J , for the complete cross-section is found by summing the torsion constants of the narrow rectangular strips which form the section. Then, from Eq. (3.29)

$$J = 2 \frac{at^3}{3} + \frac{bt^3}{3} = \frac{(2a + b)t^3}{3}$$

Therefore, from the general torsion equation (3.12)

$$\frac{d\theta}{dz} = \frac{3T}{G(2a + b)t^3} \quad (\text{i})$$

The maximum shear stress follows from Eqs (3.28) and (i), hence

$$\tau_{\max} = \pm Gt \frac{d\theta}{dz} = \pm \frac{3T}{(2a + b)t^2}.$$

Solutions to Chapter 4 Problems

S.4.1

Give the beam at D a virtual displacement δ_D as shown in Fig. S.4.1. The virtual displacements of C and B are then, respectively, $3\delta_D/4$ and $\delta_D/2$.

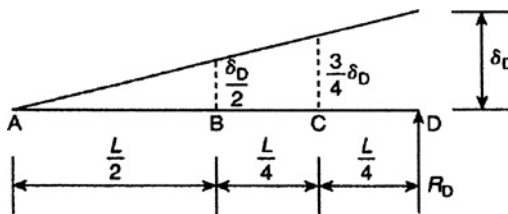


Fig. S.4.1

The equation of virtual work is then

$$R_D \delta_D - \frac{2W \delta_D}{2} - \frac{W 3\delta_D}{4} = 0$$

from which

$$R_D = 1.75W$$

It follows that

$$R_A = 1.25W.$$